

SPECIAL MOBILITY STRAND

SYSTEM IDENTIFICATION OF BRIDGES USING AMBIENT VIBRATION MEASUREMENTS AND NUMERICAL SIMULATIONS (CASE STUDIES) Damir Zenunovic Tirana, April 2019

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CASE STUDY 1 – THE BRIDGE OVER RIVER BOSNIA IN SARAJEVO





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Elevation and cross-sections of the bridge





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Test equipment





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Test equipment









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Measurement on base station 6R (middle of the span)

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Peak Displace	ement	0.284	0.0959	0.0166	mm	e	ŧ					+			
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Ambient vibration testing procedure consists of real time recording of the vibrations and processing of the records by means of the Fast Fourier Transform , i. e. obtaining of Fourier Amplitude Spectra.



 $X(f) = \int^{+\infty} x(t) e^{-2\pi i f} dt$



frequencies (Hz)



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The peaks of the amplitude spectrum occur at the predominant frequencies of the time function x(t), which due to the flat spectrum of the excitation represent the natural frequencies of the structure.





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Measurement on base station 6R (middle of the span)





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Comparison of vertical channels





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Measured dominant frequency

Measuring points	Trans. Freq. (Hz)	Vert. Freq. (Hz)	Long. Freq. (Hz)
M1	5.62, 5.63, 5.47,	5.45, 5.60, 4.98,	5.61, 5.60, 3.81,
4R, 5R, 6R, 7R, 8R	5.62, 5.63	3.81, 5.61	3.81, 3.81
M2	5.77, 5.97, 16.4,	7.69, 5.50, 4.97,	19.6, 5.43, 3.73,
2R, 3R, 6R, 9R, 10R	5.97, 6.00	5.43, 7.58	5.43, 6.16
M3	5.76, 5.75, 5.57,	7.85, 7.86, 4.95,	21.7, 5.56, 3.85,
1R, 2R, 6R, 10R, 11R	5.75, 5.75	5.73, 5.73	5.56, 5.56
M4	5.76, 5.76, 5.76,	5.38, 5.74, 5.76,	5.48, 5.48, 5.48,
6R, 4L, 5L, 6L, 7L	5.49, 5.76	5.36, 17.2	5.48, 3.88
M5	5.54, 6.48, 5.75,	5.02, 5.54, 5.78,	3.86, 5.54, 5.54,
6R, 3L, 4L, 8L, 9L	7.74, 5.02	5.02, 5.54	17.2, 5.54
M6	16.3, 6.70, 19.3,	6.85, 20.3, 6.70,	3.92, 7.95, 19.50,
6R, 1L, 2L, 10L, 11L	5.67, 8.24	8.17, 20.9	6.29, 6.29
M7	4.95, 14.1, 4.99,	4.99, 2.27, 4.99,	16.1, 5.50, 5.50,
6R, 12, 13, 16, 17	2.01, 2.00	2.00, 2.01	2.01, 2.02
M8	6.84, 16.6, 5.84,	6.84, 2.39, 17.1,	3.91, 6.28, 6.37,
6R, 12, 13, 16, 17	2.00, 2.00	2.00, 2.00	2.00, 2.00



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[dB | (1 mm/s)² / Hz]



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Average of All Spectral Densities for all Test Setups



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Modal Values

Frequency = 3.855 Hz Damping = 2.258 %

Display Settings

Rotation Horz. = 30" Rotation Vert. = 30" Translation Horz. = 0 Translation Vert. = 0 Zoom Level = 138% Amplitude = 16% Phase Angle = 270" Frames per Sec.= 0

Modal Values

Frequency = 5.342 Hz Damping = 1.656 %

Display Settings

Rotation Horz. = 30* Rotation Vert. = 30* Translation Horz. = 0 Translation Vert. = 0 Zoom Level = 138% Amplitude = 16% Phase Angle = 306* Frames per Sec.= 0





Modal Values

Frequency = 4.971 Hz Damping = 1.364 %

Display Settings

Rotation Horz. = 30* Rotation Vert. = 30* Translation Horz. = 0 Translation Vert. = 0 Zoom Level = 138% Amplitude = 16% Phase Angle = 306* Frames per Sec.= 0

Modal Values

Frequency = 6.698 Hz Damping = 2.101 %

Display Settings

Rotation Horz. = 30* Rotation Vert. = 30* Translation Horz. = 0 Translation Vert. = 0 Zoom Level = 138% Amplitude = 16% Phase Angle = 270* Frames per Sec.= 0





STUDIORUM INALA

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Transfer data to another program





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Transfer data to another program



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Finite Element Models (FEMs)





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Review of the FEMs

S1-NB1-M1	S1-NB1-M1 S		S1-NB2-M	[2	S1-NB3-M2		
S2-NB1-M1	S	2-NB1-M2	S2-NB2-M	[2	S2-NB3-M2		
S3-NB1-M1	S	3-NB1-M2	S3-NB2-M	[2	S3-NB3-M2		
S4-NB1-M1	S	4-NB1-M2	S4-NB2-M	2	S4-NB3-M2		
S5-NB1-M1	S	5-NB1-M2	S5-NB2-M	2	S5-NB3-M2		
S6-NB1-M1	S	6-NB1-M2	S6-NB2-M	2	S6-NB3-M2		
SOIL		NEOPREN EI BEAI	LASTOMERIC RING	CO	NCRETE STRENGTH		
S1- absolute stiff		NB1 – designe	d stiffness (DS)	M1 – designed strength			
S2 – 10 x k (S4)		NB2 – 1	1.5 x DS	$(\underline{\text{fck}} = 30 \text{MPa}, \underline{\text{Ecm}} = 31 \text{GPa})$			
S3 – 5 x k (S4)		NB3	- stiff	M2 – theoretical strength after 45			
S4 – empirical stiffness	of soil			years'	service (Ec0 = $38,5$ GPa)		
(k = 15000kN/m ²) has selected based on the exp with similar soils	been erience						
S5 – layered soil (LS) modulus E determined (PGSM)	with 1 by						
S6 – LS with 10% of mo	dulus E						



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Modal frequencies

	LONGITUDINAL MODE	TRANSVERSE MODE	BENDING MODE	TORSIONAL MODE
Ambient Vibration Measurements	-	3.855	4.971	5.342
FEMs				
S1-NB1-M1	2.654 (2)	2.803 (3)	3.673 (4)	3.896 (6)
S2-NB1-M1	2.442 (1)	2.454 (2)	3.654 (3)	3.810 (6)
S3-NB1-M1	2.267 (2)	2.148 (1)	3.651 (4)	3.616 (3)
S4-NB1-M1	1.567 (2)	1.444 (1)	3.630 (4)	2.967 (3)
S5-NB1-M1	2.523 (2)	2.312 (1)	3.907 (6)	3.711 (5)
S6-NB1-M1	2.459 (2)	1.969 (1)	3.884 (6)	3.405 (4)
S1-NB1-M2	2.751 (1)	<mark>3.641 (2)</mark>	3.798 (5)	4.377 (6)
S2-NB1-M2	2.512 (1)	2.522 (2)	3.828 (3)	3.949 (6)
S3-NB1-M2	2.323 (2)	2.198 (1)	3.824 (4)	3.737 (3)
S4-NB1-M2	1.586 (2)	1.448 (1)	3.794 (6)	3.030 (3)
S5-NB1-M2	2.614 (2)	2.369 (1)	4.081 (6)	3.806 (5)
S6-NB1-M2	2.542 (2)	2.011 (1)	4.054 (6)	3.483 (3)
S1-NB2-M2	2.971 (1)	<mark>3.731 (2)</mark>	3.825 (3)	4.443 (6)
S2-NB2-M2	2.666 (2)	2.556 (1)	3.860 (3)	4.065 (6)

Modal frequencies

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	LONGITUDINAL	TRANSVERSE	BENDING	TORSIONAL
	MODE	MODE	MODE	MODE
Ambient Vibration Measurements	-	3.855	4.971	5.342
FEMs				
S3-NB2-M2	2.442 (2)	2.224 (1)	3.857 (3)	3.857 (4)
S4-NB2-M2	1.627 (2)	1.467 (1)	3.827 (6)	3.124 (3)
S5-NB2-M2	2.804 (2)	2.411 (1)	4.184 (6)	4.126 (5)
S6-NB2-M2	2.715 (2)	2.046 (1)	4.156 (6)	3.755 (4)
S1-NB3-M2	3.574 (1)	<mark>4.246 (3) (fig.14a)</mark>	4.136 (2)	<mark>5.026 (5) (fig.14b)</mark>
S2-NB3-M2	3.207 (2)	2.689 (1)	4.261 (3)	4.383 (4)
S3-NB3-M2	2.915 (2)	2.317 (1)	4.253 (4)	4.143 (3)
S4-NB3-M2	1.965 (2)	1.528 (1)	4.198 (4)	3.320 (3)
S5-NB3-M2	3.407 (2)	2.513 (1)	<mark>4.639 (3)(fig.14c)</mark>	4.847 (3)
S6-NB3-M2	3.239 (2)	2.146 (1)	4.609 (4)	4.318 (3)

(n) marks in parenthesis denote modes of certain models



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Comparison of the modes parameters (mode shapes) identified by AVM and FEMs



Normalized amplitudes at selected points (levels) for each particular mode shape

 $\frac{\left|X_{i}(f_{j})\right|}{\left|X_{r}(f_{i})\right|}$ $a_{i,j} = -$



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Experimentally and mathematically identified first vertical mode shapes



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- AVM





Experimentally and mathematically identified first transverse mode shapes



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CASE STUDY 2 – THE TWIN BRIDGE "GOCE DELCEV" IN SKOPJE





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Elevation and cross-sections of the bridge





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Measurements setup





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Setup	Moveable stations	Base stations
M1	37,38	16R
M2	1,2,3,4	16R
M3	1,2,3,4	16R
M4	5,6,7,8	16R
M5	9,10,11,12	16R
M6	13,14,15	16R
M7	17,18,19,20	16R
M8	21,22,23,24	16R
M9	25,26,27,28	16R
M10	29,30,31,32	16R
M11	33,34,35,36	16R
M12	33,34,35,36	16R
M13	39,40	16R
M14	41,42	16R



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Measurement on base station 16R





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Measurement on moveable station





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Measured dominant frequency

Measuring points	1st freq. (Hz)	2 nd freq. (Hz)	3 rd freq. (Hz)
16R, 1, 2, 3, 4	1.30V, 2.00VTL, 1.98L, 1.30V, 1.98	1.98VT, 2.88L, -, 1.99VTL, -	-, 3.31L, -, 2.86L, -
16R, 5, 6, 7, 8	1.28V, 2.03VT, 2.03VT, 2.03VT,	2.03VT, 2.92V, 2.92L, 2.92L, 2.92L	4,27V, -, -, -, -
16R, 9, 10, 11, 12	1.29V, 1.29V, 1.29V, 1.30V, 1.30V	2.05VT, 2.06VT, 2.06VT, 2.05VT,	-,-, -, 2.95L, -
16R, 13, 14, 15	1.28V, 1.28V, 1.28V, 1.28V	2.02VT, 2.02VT, 2.02VT, 2.02VT	-,2.91L, -, 2.91L
16R, 17, 18, 19, 20	1.29V, 1.29V, 1.29V, 1.29V, 1.28V	2.03VT, 2.03T, 2.03VT, 2.03T, 2.03T	4,23V, 3.34L, -, 3.35L, 3.35L
16R, 21, 22, 23, 24	1.28V, 1.28V, 1.28V, 1.28V, 1.28V	2.02VT, 2.02VT, 2.02VT, 2.02VT,	-,2.90L, -, 2.91L, 2.91L
16R, 25, 26, 27, 28	1.28V, 1.28V, 1.28V, 1.28V, 1.28V	2.00VT, 2.00VTL, 2.00VT, 2.00VT,	-,2.88L,2.88L, 2.88L,2.88L
16R, 29, 30, 31, 32	1.27V, 1.80VTL, 1.80VT, 1.80V, -	2.10VT, 2.10VTL, 2.10VT, 2.10VT,	-,2.88L,2.88L, 2.91L,2.91L
16R, 33, 34, 35, 36	1.28V, 1.28V, 1.28V, 1.28L, 2.00L	2.02VT, 2.02VTL, 2.02VTL, 2.02TL,	-,2.84L, -, 2.84L, -
16R, 37, 38	1.27V, 4.20VTL, 2.00T	2.01VT, -, 2.14V	4.23V, -, 8.88L
16R, 39, 40	1.29V, 2.90L, 2.07V	1.97VT, 7.83T, 4.63T	-,13.7V,11.8L



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1st vertical mode



Modal Values

Frequency = 1.273 Hz Damping = 0.9538 %

Display Settings

Rotation Horz. = 30° Rotation Vert. = 29° Translation Horz. = 0 Translation Vert. = 0 Zoom Level = 140% Amplitude = 100% Phase Angle = 125° Frames per Sec.= 0

Undeformed Geometry



Deformed Geometry





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1st transverse mode





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2nd vertical mode



Damping = 1.094 %

Modal Values

Rotation Horz. = 30° Rotation Vert. = 29° Translation Horz. = 0 Translation Vert. = 0 Zoom Level = 140% Amplitude = 100% Phase Angle = 306° Frames per Sec.= 0

Undeformed Geometry



Deformed Geometry





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2nd transverse mode





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Finite Element Models (FEMs)





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Mathematically identified 1st vertical mode



Modes	AVM	AM3
First vertical	1.273	1.273
First transverse	2.000	1.921
Second vertical	3.235	3.542
Second transverse	4.65	5.852



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Mathematically identified 2nd transverse mode



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Experimentally and mathematically identified first vertical mode shapes





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Experimentally and mathematically identified first transverse mode shapes





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Experimentally and mathematically identified second vertical mode shapes





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Experimentally and mathematically identified second transverse mode shapes





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CASE STUDY 3 – THE CABLE - STAYED BRIDGE IN TUZLA





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Elevation and cross-sections of the bridge





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[dB|(1 m/s²)²/Hz]

Frequency Domain Decomposition - Peak Picking Average of the Normalized Singular Values of Spectral Density Matrices of all Test Setups



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1st mode







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2nd mode





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Mathematically identified 1st mode



Experimentally and mathematically identified modal frequencies (Hz)

MODES	AVM	AM4
FIRST MODE	2.719	2.727
SECOND MODE	6.790	6.512



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Mathematically identified 2nd mode



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The presented studies show that the signal analysis of ambient vibration records allows the determination of the dynamic characteristics of the bridge. In addition, the frequency and associated modes of vibration can be assessed with adequate mathematical model. The presented results clearly indicate the great potential that ambient vibration measurements hold for monitoring bridge structures. The data collected during the ambient vibration test, which only took some hours and very few resources, processed with adequate algorithms provided very useful information. The comparisons presented in case studies constitute a validation of the developed mathematical models and at the same time permit some fine tuning, especially concerning the boundary conditions and unexpected channel errors. In particular, this slides clearly shows that it was possible to extract a lot of useful information from data collected during the ambient vibration test.



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Thank you for your attention e-mail: damir.zenunovic@untz.ba

Knowledge FOr Resilient soCiEty